

Prepared By:



Stormwater Management Report

East Ridge Business Park - Blocks 3 & 4
Plan 16M-44 - Owen Sound, ON

Andpet Realty Ltd.

GMBP File: 221210

**August 2021
Revised: July 2022**

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EAST RIDGE BUSINESS PARK - BLOCKS 3 & 4**PLAN 16M-44, OWEN SOUND
STORMWATER MANAGEMENT REPORT****AUGUST 2021
REVISED: JULY 2022****GMBP FILE: 221210**

1. INTRODUCTION AND BACKGROUND

Andpet Realty Ltd. proposes to develop Blocks 3 and 4, Plan 16M-44, along 27th Avenue East, in the East Ridge Business Park located within commercial subdivision lands on the north westerly corner of 28th Avenue East and 16th Street East in the City of Owen Sound as shown in Figure No. 1. The property Owner, Andpet Realty Ltd. has requested that GM BluePlan Engineering Limited (GMBP) prepare a Stormwater Management (SWM) Report for the proposed development per the recommendation of the January 2011 Surface Water Management Report (2011 Report) prepared by GMBP (formerly Gamsby and Mannerow Limited).

The Owner proposes ultimately to develop the property with 10 storage buildings constructed across the property. This includes one two-storey building with an attached office and maintenance building on the south end of the site. Parking will along the north side of the two-storey building and office. For the purposes of this report, 27th Avenue East is assumed to be aligned in a north-south direction.

This report has been prepared by GMBP to document the design of the SWM system of the proposed development in support of Site Plan Approval.

2. EXISTING CONDITIONS AND DESIGN CRITERIA

Under existing conditions, the 2.01 ha property is undeveloped with no existing buildings. The existing conditions are as described in the 2011 Report.

Per the City of Owen Sound and the 2011 Report's individual lot requirements, the design criteria are as follows:

1. Post-development flows from the site during all storm events up to, and including, a 100-year design storm event, are expected to be attenuated to the existing condition peak flow rate of the respective storm event.
2. An "Enhanced" level of water quality treatment (80% long-term TSS Removal) is provided prior to runoff discharging from the subject property.
3. Thermal mitigation measures are employed on-site to reduce the potential temperature increase due to the proposed lot development.

3. POST-DEVELOPMENT CONDITIONS AND STORMWATER MANAGEMENT PLAN

The site is planned to be developed in phases. Ultimately, the site will have 10 storage buildings with one of the storage buildings being two-stories and having an attached office space. Proposed parking areas are located southerly side of the site, north of the two-story building and office space. The total combined area of the proposed buildings is approximately 5,600 m². The entrance to the site is proposed from 27th Avenue East at the southeast corner of the subject property. Stormwater runoff from the proposed development is to be attenuated by a stormwater management system at the north end of the site prior to draining from the site.

A 998 m² SWM pond is proposed along the northerly side of the subject property to provide stormwater attenuation for runoff from the entire site. A 125 mm diameter orifice plate is proposed on the outlet of the SWM pond to attenuate flow from the on-site storm sewer to the 18th Street East ditch. The orifice plate slowly releases runoff from the system, and the SWM pond allows stormwater to temporarily pond within the voids of the stone, until it can slowly drain from the site. Given the existing soil conditions it is unlikely that infiltration will provide impactful drainage from the site.

An emergency overflow weir is proposed for runoff to spill overland to the 18th Street East ditch under emergency situations only.

4. QUANTITY CONTROL CRITERIA, PARAMETERS AND MODELLING

4.1 Design Rainfall Events

The City of Owen Sound Engineering Standards provide rainfall data for design storms in the form of Chicago Storm Parameters. The Chicago storm input parameters used to model the various design rainfall events for the subject property are summarized in the following Table 1.

Table 1 - Design Rainfall Events – Provided by the City of Owen Sound Engineering Standards

Coefficient	1:2-Year	1:5-Year	1:25-Year	1:100-Year
A	854.100	1234.576	1750.276	2171.754
B	7.781	8.297	8.303	8.303
C	0.830	0.851	0.862	0.867
R	0.375	0.375	0.375	0.375
Duration (min)	180	180	210	210
Depth (mm)	33.228	42.929	59.007	71.271
Intensity (mm/hr)	101.673	134.692	165.718	202.862

4.2 Site Soil Conditions

The soil type within the subject property is characterized as Harkaway Silt Loam which consists of silt loam over brown loam over brown clay over calcareous till, as per the Grey County Soils Map (Ontario Soil Survey Report No. 17) published by the Department of Agriculture. The Harkaway Silt Loam soil type is known to be of the Hydrological Soil Group (HSG) BC. These soils generally have properties not conducive to infiltration.

An SCS Curve Number of 71 for unimproved lands on the site was used based on Design Chart 1.09: Soil/Land Use Curve Numbers provided by the US Department of Agriculture, 1972. Impervious surfaces within the subject property are associated with an SCS Curve Number of 98.

4.3 Existing and Post-Development Catchment Areas

The entire site under existing conditions was modelled as Catchment 10 with an imperviousness of 0% for the entire subject area. It is worth noting that the existing conditions reflect the catchment area and drainage outlet information provided in the 2011 Report. Runoff from the entire site generally drains from southeast to northwest towards the 18th Street East Ditch.

Under proposed conditions, the site was modelled as one catchment area (Catchment 100) with an imperviousness of 75%, based on the overall proposed site conditions. There are several catch basins proposed throughout the site to drain runoff through on-site storm sewers to the proposed stormwater management pond that will attenuate post-development peak flows to less than existing condition peak flows.

The existing and proposed condition drainage areas are shown on Figure No. 2. The existing and proposed conditions catchments are outlined in Table 2 below.

The Stage-Storage-Discharge design calculations are attached as Appendix A and MIDUSS computer modelling is attached as Appendix B.

The results of the routing analysis are summarized in Section 4.4.

Table 2 – Existing and Proposed Condition Catchments

Catchment	Description	Area (ha)	Impervious Level (%)
10	Existing Conditions – Entire Property	2.01	0
100	Proposed Conditions – Entire Property	2.01	75

4.4 MIDUSS Quantity Control Modelling Results

MIDUSS modelling software was used to model the expected post-development conditions stormwater runoff from the subject property during the various design storm events. Results from the models are summarized in the following tables, and the modelling is provided for reference in Appendix B.

Table 3 below also provides the total peak flow rates discharging from the subject property under post-development conditions as modelled in MIDUSS.

Table 3: Comparison of Allowable and Post-Development Peak Flow Runoff Rates

Storm Return Period (year)	1:2	1:5	1:25	1:100
Pro-Rated Allowable Peak Flow Rate (m ³ /s)	0.008	0.021	0.054	0.091
Post-Development Peak Flow Rates (m ³ /s)	0.006	0.013	0.039	0.041

As shown in Table 3 above, under post-development conditions, the peak flow discharge rates from the proposed SWM pond would be less than the allowable release rates (existing conditions) for all design storm events up to, and including, the 1:100-year design storm event.

Table 4 below summarizes the capacity available at the various stages in the stormwater management pond and provides a comparison to the capacity that is expected to be used during the various design storm events.

Table 4: Stage-Storage-Discharge Capacities for Stormwater Management Pond

	Available Capacity in SWM Facility Design			Capacity Used During Various Design Storm Events		
	Peak Flow (m³/s)	Storage Volume (m³)	Storage Elevation (m)	Peak Flow (m³/s)	Storage Volume (m³)	Storage Elevation (m)
Bottom of Stone Trench	0.000	0	223.95	---	---	---
C/L of First Row of Perforations on Vertical Riser Outlet (First Controlled Outlet)	0.000	11.0	224.10			
Top of Stone/Bottom of Pond	0.004	56.0	224.50			
10 mm "First Flush"				0.004	81.6	224.54
1:2-year Design Storm				0.006	395.4	224.77
DICB L/S Grate	0.006	521.1	224.85			
1:5-year Design Storm	---	---	---	0.013	536.4	224.86
1:25-year Design Storm	---	---	---	0.039	658.4	224.94
1:100-year Design Storm	---	---	---	0.041	828.7	225.05
Overflow Weir	0.041	837.0	225.05	---	---	---
Top of Bank	0.167	998.0	225.15	---	---	---

As shown in Table 4 above, during all design storm events up to, and including, the 1:100-year return, the runoff is expected to discharge via the orifice outlet without spilling via the overflow weir.

5. STORMWATER QUALITY TREATMENT

Based on the requirements from the City of Owen Sound, the on-site quality control for the stormwater flow is to achieve an 80% long term total suspended solids (TSS) removal rate while treating 90% of the annual runoff, prior to release to the off-site receiving drainage system.

The post-development site, Catchment 100, features primarily paved areas bordered with grassed areas. Stormwater quality treatment for Catchment 100 runoff is provided by the SWM facility. The facility features a channelized section of clear stone, proposed to provide additional TSS removal. All inflows conveyed through the channelized section of the SWM facility during a minor storm event (<10 mm rainfall depth) are proposed to flow through the clear stone prior to reaching the outlet. The clear stone will slow the flow through the facility to encourage sediment to settle out, and to provide treatment during minor storm events by directing runoff through it prior to discharging from the proposed SWM facility.

As shown in Table 4, all flow to the SWM facility is conveyed through the clear stone to the perforated vertical riser pipe during a 2-year design storm event.

For a site with at least 85% imperviousness, and when using a dry pond for water quality treatment, the Ministry of the Environment and Climate Change Stormwater Management Planning and Design Manual (MOECC SWMPD Manual) recommends 240 m³/ha of stormwater storage capacity to meet 60% long-term TSS removal. Based on this requirement, the proposed detention area should have a minimum volume of approximately 482 m³ of capacity. The proposed stormwater detention facility has a storage volume of 837 m³ at the overflow weir.

It is expected that this process of treatment with the pond volume over the requirements and the channelized clear stone will meet the MOECC enhanced level of 80% long-term TSS removal.

6. STORMWATER TEMPERATURE MITIGATION

Temperature mitigation measures are required due to the temperature increase often associated with SWM facilities as is outlined in section 4.4 of the 2011 Report. The measures outlined below are proposed to be implemented to reduce the impact of temperature increase on the stormwater draining from the site.

While planting vegetation along the edges of the SWM pond to increase shade may help to reduce the temperature of the stormwater, the main method of mitigation is provided from the channel of clear stone located within the stormwater management facility. From Section 4.4 of the SWMPDG, “treatment of water, by routing the discharge through a subsurface trench filled with clear stone, has also been suggested to reduce temperature”, and that, “the trench should be designed to accommodate [the conveyance of peak runoff rate during] frequent events (i.e., <10 mm) which will have a greater effect on the thermal regime of the receiving water”. As discussed in Section 5.1, all runoff draining to the SWM facility is proposed to be conveyed through the channelized section of clear stone prior to draining to the outlet system. Therefore, the channelized sections of clear stone are expected to provide thermal cooling effects to inflows as heat is transferred from the runoff to the clear stone.

7. SUMMARY

The proposed development includes the construction of ten (10) storage buildings totalling approximately 5,600 m² and parking areas. The site is an approximately 2.01 ha property located on blocks 3 & 4 in the East Ridge Business Park in the City of Owen Sound.

Upon completion of the proposed development;

1. The proposed drainage outlet is the 18th Street East ditch draining westerly via the SWM Pond.
2. Post-development flows from the site during all storm events up to, and including, a 100-year design storm event, are expected to be attenuated to the existing condition peak flow rate of the respective storm event.
3. An Emergency Overland spill route is provided to direct runoff to the 18th Street East ditch during storm events in excess of a 100-year design storm event, without spilling to neighbouring properties or reaching the finished floor elevation of any of the proposed buildings.
4. An “Enhanced” level of water quality treatment (80% TSS Removal) is provided by a clear stone channel in the SWM Pond and exceeding the pond volume requirement of the MOECC prior to discharging from the subject property.

All of which is respectfully submitted,

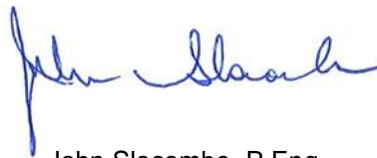
GM BLUEPLAN ENGINEERING LIMITED

Prepared by:

Reviewed by:

A handwritten signature in blue ink that reads 'Ethan Webb'.

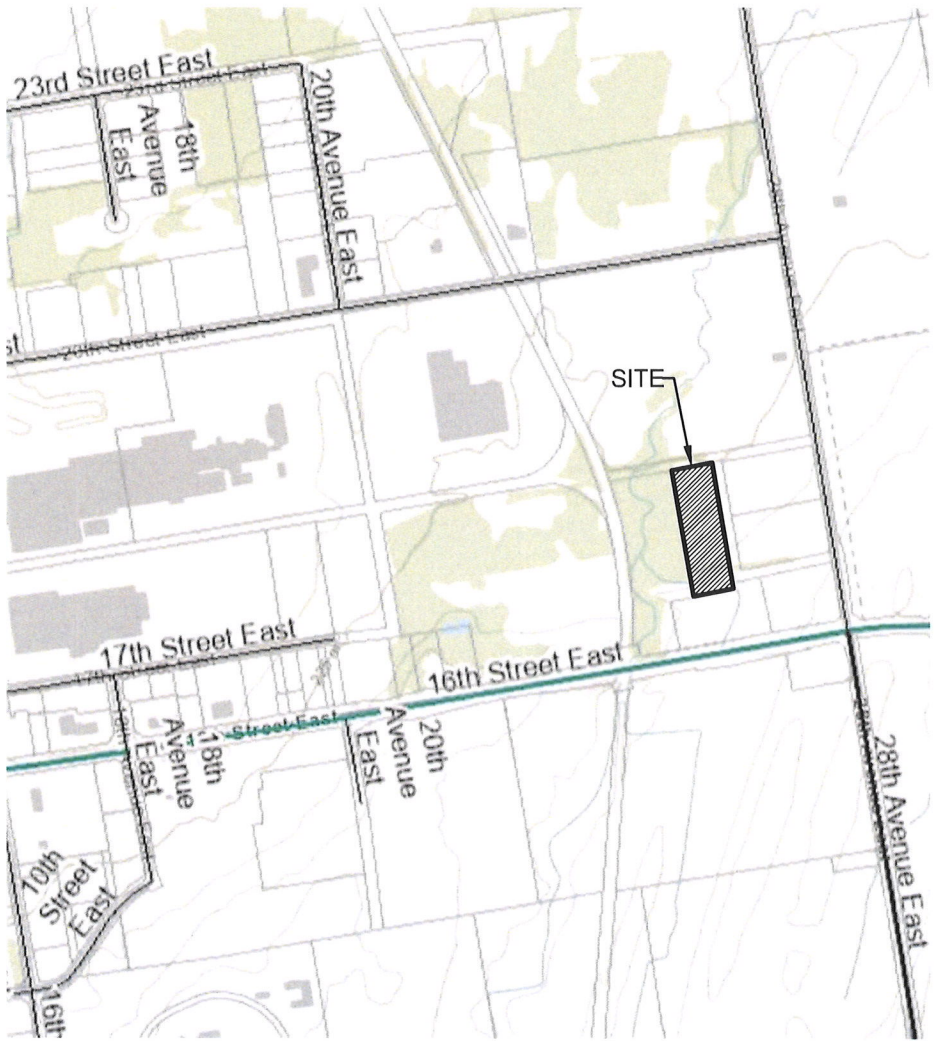
Ethan C.J. Webb, P.Eng.

A handwritten signature in blue ink that reads 'John Slocombe'.

John Slocombe, P.Eng.

FIGURES:

221210
Blocks 3 & 4 Development
East Ridge Business Park



SCALE = 1:750
JULY 2022

SITE LOCATION MAP

Andpet Realty
Blocks 3 & 4, Plan 16M-44
City of Owen Sound




Figure No. 1



221210
Blocks 3 & 4 Development
East Ridge Business Park



LEGEND

-  DRAINAGE BOUNDARY
-  EXISTING/PROPOSED CATCHMENT NUMBER
CATCHMENT AREA
PERCENT IMPERVIOUS
-  DIRECTION OF SURFACE FLOW

SCALE = 1:1,000
JULY 2022

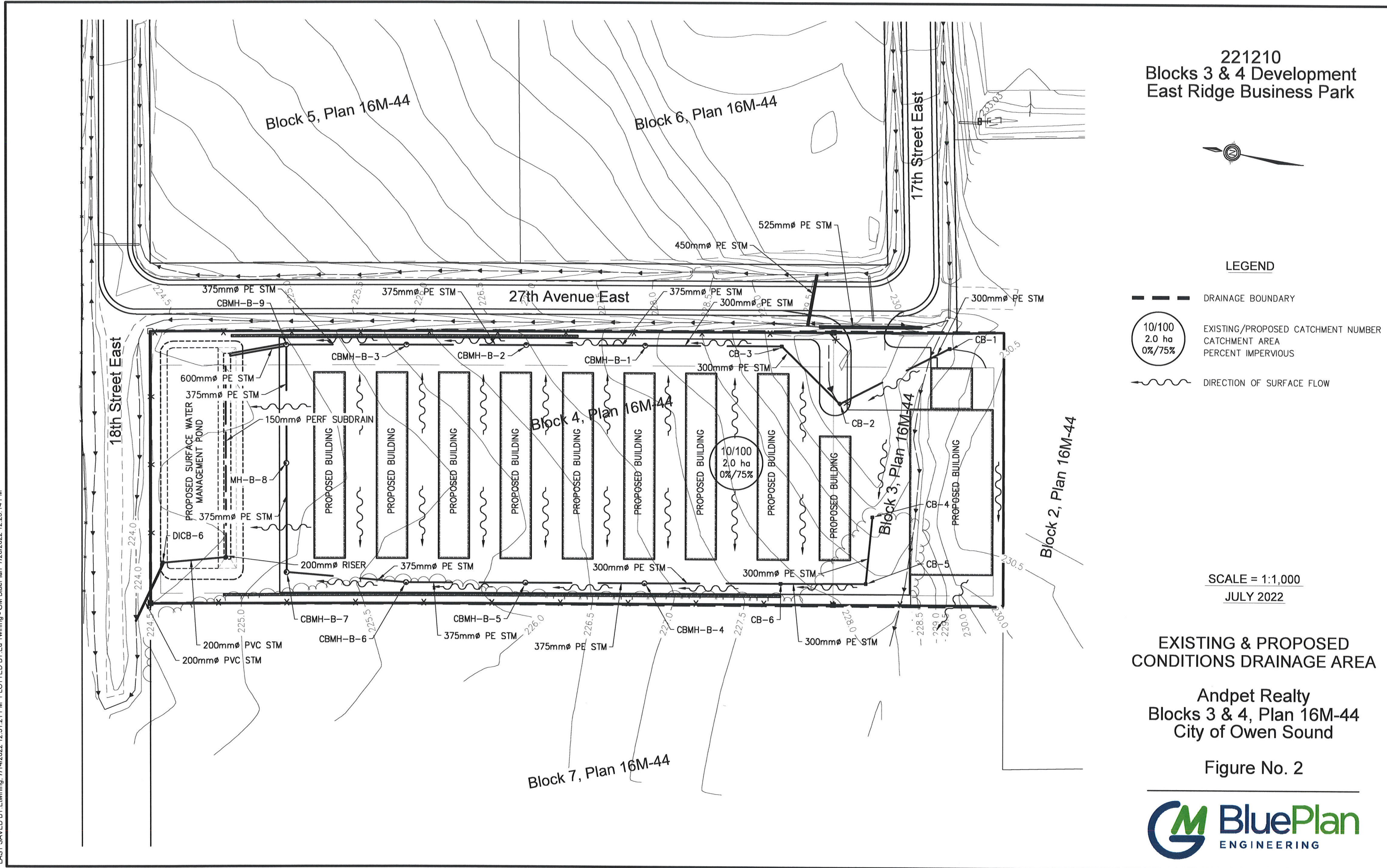
EXISTING & PROPOSED
CONDITIONS DRAINAGE AREA

Andpet Realty
Blocks 3 & 4, Plan 16M-44
City of Owen Sound

Figure No. 2



FILE: C:\Drawings\221210spe6.dwg LAYOUT: FIG 2
LAST SAVED BY: Etwinning, 7/14/2022 12:31:21 PM PLOTTED BY: Ed Twinning - GM BluePlan 7/20/2022 12:23:14 PM



**APPENDIX A:
STAGE-STORAGE-DISCHARGE CALCULATIONS**

**Block 4 - East Ridge Business Park
City of Owen Sound
Our File: 221210
July 2022**

STORAGE VOLUME CALCULATIONS

ELEV	INC D	STONE SURFACE AREA	STONE AVERAGE AREA	INC. STONE STORAGE VOL	OPEN STORAGE AREA	AVERAGE OPEN STORAGE AREA	INC. OPEN STORAGE VOL	ACCUM. VOLUME	
(m)	(m)	(m ²)	(m ²)	(m ³)	(m ²)	(m ²)	(m ³)	(m ³)	
223.90	0.00	120.80	0.00	0.00	0	0	0	0	B/ Stone Trench
223.95	0.05	120.80	121	2.4	0	0	0	0	
224.00	0.10	175.71	148	3.0	0	0	0	3	
224.05	0.15	194.01	185	3.7	0	0	0	7	
224.10	0.20	212.32	203	4.1	0	0	0	11	Perforation
224.15	0.25	230.62	221	4.4	0	0	0	15	
224.20	0.30	248.92	240	4.8	0	0	0	20	Perforation
224.25	0.35	267.22	258	5.2	0	0	0	25	
224.30	0.40	285.53	276	5.5	0	0	0	31	Perforation
224.35	0.45	303.83	295	5.9	0	0	0	37	
224.40	0.50	322.13	313	6.3	0	0	0	43	Perforation
224.45	0.55	340.44	331	6.6	0	0	0	49	
224.50	0.60	358.74	350	7.0	0	0	0	56	Top of Stone/B Pond
224.55	0.65	0.00	0	0.0	1349	675	34	90	
224.60	0.70	0	0	0.0	1377	1363	68	158	
224.65	0.75	0	0	0.0	1406	1392	70	228	
224.70	0.80	0	0	0.0	1435	1420	71	299	
224.75	0.85	0	0	0.0	1464	1449	72	371	
224.80	0.90	0	0	0.0	1493	1478	74	445	
224.85	0.95	0	0	0.0	1522	1508	75	521	
224.90	1.00	0	0	0.0	1552	1537	77	598	T/G Secondary Outlet
224.95	1.05	0	0	0.0	1582	1567	78	676	
225.00	1.10	0	0	0.0	1612	1597	80	756	
225.05	1.15	0	0	0.0	1643	1628	81	837	Weir
225.10	1.20	0	0	0.0	1674	1658	83	920	Overflow
225.15	1.25	0	0	0.0	1454	1564	78	998	Overflow

PERFORATED RISER OUTLET

Vertical Perforated 200 mm Riser
 Area based on 6 perf. per 0.10 m of vertical pipe
 Perforation Diameter = 1.27 cm (0.5")
 Cd = 0.6
 Area of Each Perf. = 0.0001267 m²
 Starting Elevation = 224.10 m
 Ending Elevation = 224.40 m
 Top of Solid Cap = 224.50 m

SECONDARY OUTLET PIPE

Orifice Dia. = 125 mm
 Orifice Area = 0.012 m²
 Coefficient = 0.6
 Invert Elev = 223.90

WEIR CALCULATIONS

d1 = 225.15 m
 h = 225.05 m
 H = 0.10 m
 2g = 19.612
 L = 2 m

STAGE-STORAGE-DISCHARGE CALCULATIONS

ELEV	STAGE	STORAGE	PERF RISER OUTLET FLOW	SECONDARY OUTLET FLOW	WEIR OUTLET FLOW	TOTAL OUTLET FLOW	
(m)	(m)	(m ³)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	
223.90	0.00	0	0.0000	0.000	0.000	0.000	B/ Stone Trench
223.95	0.05	0	0.0000	0.000	0.000	0.000	
224.00	0.05	3	0.0000	0.000	0.000	0.000	
224.05	0.10	7	0.0000	0.000	0.000	0.000	
224.10	0.15	11	0.0000	0.000	0.000	0.000	Perforation
224.15	0.20	15	0.0005	0.000	0.000	0.000	
224.20	0.25	20	0.0006	0.000	0.000	0.001	Perforation
224.25	0.30	25	0.0012	0.000	0.000	0.001	
224.30	0.35	31	0.0015	0.000	0.000	0.002	Perforation
224.35	0.40	37	0.0022	0.000	0.000	0.002	
224.40	0.45	43	0.0026	0.000	0.000	0.003	Perforation
224.45	0.50	49	0.0034	0.000	0.000	0.003	
224.50	0.55	56	0.0039	0.000	0.000	0.004	Top of Stone/B Pond
224.55	0.60	90	0.0043	0.000	0.000	0.004	
224.60	0.65	158	0.0047	0.000	0.000	0.005	
224.65	0.70	228	0.0051	0.000	0.000	0.005	
224.70	0.75	299	0.0054	0.000	0.000	0.005	
224.75	0.80	371	0.0057	0.000	0.000	0.006	
224.80	0.85	445	0.0060	0.000	0.000	0.006	
224.85	0.90	521	0.0062	0.000	0.000	0.006	T/G Secondary Outlet
224.90	0.95	598	0.0065	0.032	0.000	0.038	
224.95	1.00	676	0.0067	0.032	0.000	0.039	
225.00	1.05	756	0.0070	0.033	0.000	0.040	
225.05	1.10	837	0.0072	0.034	0.000	0.041	Weir
225.10	1.15	920	0.0074	0.035	0.041	0.083	Overflow
225.15	1.20	998	0.0076	0.036	0.124	0.167	Overflow

INSTITUTIONAL
RESEARCH CENTER

**APPENDIX B:
MIDUSS MODELLING**

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        \\os-2012r2\Users_Private\ewebb\Documents\
"                                          MIDUSS\221210\July 2022"
"          Output filename:                   Ex. and Post 1 (10mm Flush) yr.out"
"          Licensee name:                     gmbp"
"          Company                             "
"          Date & Time last used:             7/18/2022 at 8:41:29 AM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          60.000 Max. Storm length"
"          360.000 Max. Hydrograph"
" 32      STORM Canada AES"
"          4 Canada AES"
"          10.000 Rainfall depth"
"          60.000 Duration"
"          21.000 Time to peak"
"          7.000 Decay factor"
"          Maximum intensity                   31.121 mm/hr"
"          Total depth                         10.000 mm"
"          5 10hyd Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          10 Pre "
"          0.000 % Impervious"
"          2.010 Total Area"
"          200.000 Flow length"
"          2.900 Overland Slope"
"          2.010 Pervious Area"
"          200.000 Pervious length"
"          2.900 Pervious slope"
"          0.000 Impervious Area"
"          200.000 Impervious length"
"          2.900 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious SCS Curve No."
"          0.001 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          8.467 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.000      0.000      0.000      0.000 c.m/sec"
"          Catchment 10      Pervious      Impervious      Total Area "

```

"	Surface Area	2.010	0.000	2.010	hectare"
"	Time of concentration	494.543	11.147	494.236	minutes"
"	Time to Centroid	251.502	35.181	251.364	minutes"
"	Rainfall depth	10.000	10.000	10.000	mm"
"	Rainfall volume	201.00	0.00	201.00	c.m"
"	Rainfall losses	9.990	3.878	9.990	mm"
"	Runoff depth	0.010	6.122	0.010	mm"
"	Runoff volume	0.19	0.00	0.19	c.m"
"	Runoff coefficient	0.001	0.000	0.001	"
"	Maximum flow	0.000	0.000	0.000	c.m/sec"
" 40	HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"			
"		0.000	0.000	0.000	0.000"
" 33	CATCHMENT 100"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	100	Post"			
"	75.000	% Impervious"			
"	2.010	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.503	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	1.507	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious SCS Curve No."			
"	0.003	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.467	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.596	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.095	0.000	0.000	0.000 c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	0.503	1.507	2.010	hectare"
"	Time of concentration	138.874	3.130	3.330	minutes"
"	Time to Centroid	154.799	27.218	27.406	minutes"
"	Rainfall depth	10.000	10.000	10.000	mm"
"	Rainfall volume	50.25	150.75	201.00	c.m"
"	Rainfall losses	9.974	4.035	5.520	mm"
"	Runoff depth	0.026	5.965	4.480	mm"
"	Runoff volume	0.13	89.92	90.05	c.m"
"	Runoff coefficient	0.003	0.596	0.448	"
"	Maximum flow	0.000	0.095	0.095	c.m/sec"


```

" 40          HYDROGRAPH Add Runoff "
"            4  Add Runoff "
"              0.095      0.095      0.000      0.000"
" 54          POND DESIGN"
"            0.095  Current peak flow    c.m/sec"
"            0.040  Target outflow      c.m/sec"
"            90.1   Hydrograph volume    c.m"
"            26.   Number of stages"
"           223.900 Minimum water level  metre"
"           225.150 Maximum water level  metre"
"           223.900 Starting water level  metre"
"            0     Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"           223.900      0.000      0.000"
"           223.950  1.01E-05  1.01E-05"
"           224.000  2.01E-05   2.965"
"           224.050  3.01E-05   6.662"
"           224.100  4.01E-05  10.726"
"           224.150   0.00050   15.155"
"           224.200   0.00060   19.950"
"           224.250   0.00120   25.112"
"           224.300   0.00150   30.639"
"           224.350   0.00220   36.533"
"           224.400   0.00260   42.792"
"           224.450   0.00340   49.418"
"           224.500   0.00390   56.410"
"           224.550   0.00430   90.135"
"           224.600   0.00470  158.292"
"           224.650   0.00510  227.871"
"           224.700   0.00540  298.882"
"           224.750   0.00570  371.339"
"           224.800   0.00600  445.252"
"           224.850   0.00620  520.635"
"           224.900   0.03810  597.500"
"           224.950   0.03910  675.859"
"           225.000   0.04020  755.723"
"           225.050   0.04120  837.105"
"           225.100   0.08320  920.017"
"           225.150   0.1672   998.211"
"           Peak outflow          0.004   c.m/sec"
"           Maximum level        224.537  metre"
"           Maximum storage       81.632   c.m"
"           Centroidal lag        4.883   hours"
"           0.095      0.095      0.004      0.000 c.m/sec"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        \\os-2012r2\Users_Private\ewebb\Documents\
"                                          MIDUSS\221210\July 2022"
"          Output filename:                   Ex. and Post 2 yr.out"
"          Licensee name:                     gmbp"
"          Company                            "
"          Date & Time last used:             7/18/2022 at 8:37:37 AM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          360.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          854.100 Coefficient A"
"          7.781 Constant B"
"          0.830 Exponent C"
"          0.375 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                   101.673 mm/hr"
"          Total depth                         33.228 mm"
"          6 002hyd Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          10 Entire Site - Existing Conditions"
"          0.000 % Impervious"
"          2.010 Total Area"
"          200.000 Flow length"
"          2.900 Overland Slope"
"          2.010 Pervious Area"
"          200.000 Pervious length"
"          2.900 Pervious slope"
"          0.000 Impervious Area"
"          200.000 Impervious length"
"          2.900 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          71.000 Pervious SCS Curve No."
"          0.117 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          10.375 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"

```

"		0.008	0.000	0.000	0.000	c.m/sec"
"	Catchment 10		Pervious	Impervious	Total Area	"
"	Surface Area	2.010		0.000	2.010	hectare"
"	Time of concentration	92.403		6.399	92.403	minutes"
"	Time to Centroid	197.322		94.221	197.322	minutes"
"	Rainfall depth	33.228		33.228	33.228	mm"
"	Rainfall volume	667.87		0.00	667.88	c.m"
"	Rainfall losses	29.341		5.213	29.341	mm"
"	Runoff depth	3.886		28.015	3.886	mm"
"	Runoff volume	78.11		0.00	78.11	c.m"
"	Runoff coefficient	0.117		0.000	0.117	"
"	Maximum flow	0.008		0.000	0.008	c.m/sec"
" 40	HYDROGRAPH Start - New Tributary"					
"	2	Start - New Tributary"				
"		0.008	0.000	0.000	0.000	"
" 33	CATCHMENT 100"					
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	100	Entire Site - Post-Dev."				
"	75.000	% Impervious"				
"	2.010	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.503	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.507	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	71.000	Pervious SCS Curve No."				
"	0.124	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	10.375	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.839	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.305	0.000	0.000	0.000	c.m/sec"
"	Catchment 100		Pervious	Impervious	Total Area	"
"	Surface Area	0.503		1.507	2.010	hectare"
"	Time of concentration	25.948		1.797	2.932	minutes"
"	Time to Centroid	132.813		87.255	89.396	minutes"
"	Rainfall depth	33.228		33.228	33.228	mm"
"	Rainfall volume	166.97		500.91	667.88	c.m"
"	Rainfall losses	29.106		5.365	11.300	mm"
"	Runoff depth	4.122		27.863	21.928	mm"
"	Runoff volume	20.71		420.03	440.75	c.m"

```

"          Runoff coefficient      0.124      0.839      0.660      "
"          Maximum flow            0.005      0.305      0.305      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.305      0.305      0.000      0.000"
" 54      POND DESIGN"
"          0.305  Current peak flow  c.m/sec"
"          0.046  Target outflow   c.m/sec"
"          440.7  Hydrograph volume  c.m"
"          26.    Number of stages"
"          223.900  Minimum water level  metre"
"          225.150  Maximum water level  metre"
"          223.900  Starting water level  metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"          223.900      0.000      0.000"
"          223.950  1.01E-05  1.01E-05"
"          224.000  2.01E-05   2.965"
"          224.050  3.01E-05   6.662"
"          224.100  4.01E-05  10.726"
"          224.150   0.00050   15.155"
"          224.200   0.00060   19.950"
"          224.250   0.00120   25.112"
"          224.300   0.00150   30.639"
"          224.350   0.00220   36.533"
"          224.400   0.00260   42.792"
"          224.450   0.00340   49.418"
"          224.500   0.00390   56.410"
"          224.550   0.00430   90.135"
"          224.600   0.00470  158.292"
"          224.650   0.00510  227.871"
"          224.700   0.00540  298.882"
"          224.750   0.00570  371.339"
"          224.800   0.00600  445.252"
"          224.850   0.00620  520.635"
"          224.900   0.03810  597.500"
"          224.950   0.03910  675.859"
"          225.000   0.04020  755.723"
"          225.050   0.04120  837.105"
"          225.100   0.08320  920.017"
"          225.150   0.1672   998.211"
"          Peak outflow            0.006      c.m/sec"
"          Maximum level           224.766      metre"
"          Maximum storage         395.411      c.m"
"          Centroidal lag          14.158      hours"
"              0.305      0.305      0.006      0.000 c.m/sec"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        \\os-2012r2\Users_Private\ewebb\Documents\
"                                          MIDUSS\221210\July 2022"
"          Output filename:                   Ex. and Post 5 yr.out"
"          Licensee name:                     gmbp"
"          Company                            "
"          Date & Time last used:            7/18/2022 at 8:33:19 AM"
" 31          TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          360.000 Max. Hydrograph"
" 32          STORM Chicago storm"
"          1 Chicago storm"
"          1234.580 Coefficient A"
"          8.297 Constant B"
"          0.851 Exponent C"
"          0.375 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                   134.693 mm/hr"
"          Total depth                         42.929 mm"
"          6 005hyd Hydrograph extension used in this file"
" 33          CATCHMENT 10"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          10 Entire Site - Existing Conditions"
"          0.000 % Impervious"
"          2.010 Total Area"
"          200.000 Flow length"
"          2.900 Overland Slope"
"          2.010 Pervious Area"
"          200.000 Pervious length"
"          2.900 Pervious slope"
"          0.000 Impervious Area"
"          200.000 Impervious length"
"          2.900 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          71.000 Pervious SCS Curve No."
"          0.177 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          10.375 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"

```

"		0.021	0.000	0.000	0.000 c.m/sec"	
"	Catchment 10		Pervious	Impervious	Total Area	"
"	Surface Area	2.010		0.000	2.010	hectare"
"	Time of concentration	69.279		5.646	69.278	minutes"
"	Time to Centroid	173.873		91.776	173.873	minutes"
"	Rainfall depth	42.929		42.929	42.929	mm"
"	Rainfall volume	862.87		0.00	862.87	c.m"
"	Rainfall losses	35.340		5.350	35.340	mm"
"	Runoff depth	7.589		37.579	7.589	mm"
"	Runoff volume	152.54		0.00	152.54	c.m"
"	Runoff coefficient	0.177		0.000	0.177	"
"	Maximum flow	0.021		0.000	0.021	c.m/sec"
" 40	HYDROGRAPH Start - New Tributary"					
"	2	Start - New Tributary"				
"		0.021	0.000	0.000	0.000"	
" 33	CATCHMENT 100"					
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	100	Entire Site - Post-Dev."				
"	75.000	% Impervious"				
"	2.010	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.503	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.507	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	71.000	Pervious SCS Curve No."				
"	0.181	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	10.375	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.867	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.426	0.000	0.000	0.000 c.m/sec"	
"	Catchment 100		Pervious	Impervious	Total Area	"
"	Surface Area	0.503		1.507	2.010	hectare"
"	Time of concentration	19.454		1.585	2.747	minutes"
"	Time to Centroid	121.159		85.654	87.961	minutes"
"	Rainfall depth	42.929		42.929	42.929	mm"
"	Rainfall volume	215.72		647.15	862.87	c.m"
"	Rainfall losses	35.166		5.695	13.063	mm"
"	Runoff depth	7.763		37.234	29.866	mm"
"	Runoff volume	39.01		561.30	600.31	c.m"

"	Runoff coefficient	0.181	0.867	0.696	"
"	Maximum flow	0.012	0.425	0.426	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.426	0.426	0.000	0.000"
" 54	POND DESIGN"				
"	0.426 Current peak flow				c.m/sec"
"	0.017 Target outflow				c.m/sec"
"	600.3 Hydrograph volume				c.m"
"	26. Number of stages"				
"	223.900 Minimum water level				metre"
"	225.150 Maximum water level				metre"
"	223.900 Starting water level				metre"
"	0 Keep Design Data: 1 = True; 0 = False"				
"	Level Discharge				Volume"
"	223.900	0.000			0.000"
"	223.950	1.01E-05			1.01E-05"
"	224.000	2.01E-05			2.965"
"	224.050	3.01E-05			6.662"
"	224.100	4.01E-05			10.726"
"	224.150	0.00050			15.155"
"	224.200	0.00060			19.950"
"	224.250	0.00120			25.112"
"	224.300	0.00150			30.639"
"	224.350	0.00220			36.533"
"	224.400	0.00260			42.792"
"	224.450	0.00340			49.418"
"	224.500	0.00390			56.410"
"	224.550	0.00430			90.135"
"	224.600	0.00470			158.292"
"	224.650	0.00510			227.871"
"	224.700	0.00540			298.882"
"	224.750	0.00570			371.339"
"	224.800	0.00600			445.252"
"	224.850	0.00620			520.635"
"	224.900	0.03810			597.500"
"	224.950	0.03910			675.859"
"	225.000	0.04020			755.723"
"	225.050	0.04120			837.105"
"	225.100	0.08320			920.017"
"	225.150	0.1672			998.211"
"	Peak outflow		0.013		c.m/sec"
"	Maximum level		224.860		metre"
"	Maximum storage		536.375		c.m"
"	Centroidal lag		16.676		hours"
"		0.426	0.426	0.013	0.000 c.m/sec"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        \\os-2012r2\Users_Private\ewebb\Documents\
"                                              MIDUSS\221210\July 2022"
"          Output filename:                   Ex. and Post 25 yr.out"
"          Licensee name:                     gmbp"
"          Company                            "
"          Date & Time last used:             7/18/2022 at 8:28:23 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          210.000 Max. Storm length"
"          360.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1750.276 Coefficient A"
"          8.303  Constant B"
"          0.862  Exponent C"
"          0.375  Fraction R"
"          210.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                   165.717  mm/hr"
"          Total depth                         59.007  mm"
"          6  025hyd Hydrograph extension used in this file"
" 33      CATCHMENT 10"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          10  Entire Site - Existing Conditions"
"          0.000  % Impervious"
"          2.010  Total Area"
"          200.000 Flow length"
"          2.900  Overland Slope"
"          2.010  Pervious Area"
"          200.000 Pervious length"
"          2.900  Pervious slope"
"          0.000  Impervious Area"
"          200.000 Impervious length"
"          2.900  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          71.000 Pervious SCS Curve No."
"          0.258  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          10.375 Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```


"		0.054	0.000	0.000	0.000 c.m/sec"	
"	Catchment 10		Pervious	Impervious	Total Area	"
"	Surface Area	2.010		0.000	2.010	hectare"
"	Time of concentration	49.499		5.160	49.499	minutes"
"	Time to Centroid	168.462		102.651	168.461	minutes"
"	Rainfall depth	59.007		59.007	59.007	mm"
"	Rainfall volume	1186.04		0.00	1186.05	c.m"
"	Rainfall losses	43.792		5.436	43.792	mm"
"	Runoff depth	15.215		53.571	15.215	mm"
"	Runoff volume	305.82		0.00	305.82	c.m"
"	Runoff coefficient	0.258		0.000	0.258	"
"	Maximum flow	0.054		0.000	0.054	c.m/sec"
" 40	HYDROGRAPH Start - New Tributary"					
"	2	Start - New Tributary"				
"		0.054	0.000	0.000	0.000"	
" 33	CATCHMENT 100"					
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	100	Entire Site - Post-Dev."				
"	75.000	% Impervious"				
"	2.010	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.503	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.507	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	71.000	Pervious SCS Curve No."				
"	0.262	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	10.375	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.896	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.535	0.000	0.000	0.000 c.m/sec"	
"	Catchment 100		Pervious	Impervious	Total Area	"
"	Surface Area	0.503		1.507	2.010	hectare"
"	Time of concentration	13.900		1.449	2.557	minutes"
"	Time to Centroid	127.186		97.087	99.765	minutes"
"	Rainfall depth	59.007		59.007	59.007	mm"
"	Rainfall volume	296.51		889.53	1186.05	c.m"
"	Rainfall losses	43.521		6.164	15.503	mm"
"	Runoff depth	15.487		52.844	43.504	mm"
"	Runoff volume	77.82		796.62	874.44	c.m"

```

"          Runoff coefficient      0.262      0.896      0.737      "
"          Maximum flow            0.031      0.533      0.535      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"                0.535      0.535      0.000      0.000"
" 54      POND DESIGN"
"          0.535  Current peak flow  c.m/sec"
"          0.356  Target outflow   c.m/sec"
"          874.4  Hydrograph volume  c.m"
"          26.    Number of stages"
"          223.900  Minimum water level  metre"
"          225.150  Maximum water level  metre"
"          223.900  Starting water level  metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"                Level Discharge  Volume"
"          223.900      0.000      0.000"
"          223.950  1.01E-05  1.01E-05"
"          224.000  2.01E-05  2.965"
"          224.050  3.01E-05  6.662"
"          224.100  4.01E-05  10.726"
"          224.150  0.00050  15.155"
"          224.200  0.00060  19.950"
"          224.250  0.00120  25.112"
"          224.300  0.00150  30.639"
"          224.350  0.00220  36.533"
"          224.400  0.00260  42.792"
"          224.450  0.00340  49.418"
"          224.500  0.00390  56.410"
"          224.550  0.00430  90.135"
"          224.600  0.00470  158.292"
"          224.650  0.00510  227.871"
"          224.700  0.00540  298.882"
"          224.750  0.00570  371.339"
"          224.800  0.00600  445.252"
"          224.850  0.00620  520.635"
"          224.900  0.03810  597.500"
"          224.950  0.03910  675.859"
"          225.000  0.04020  755.723"
"          225.050  0.04120  837.105"
"          225.100  0.08320  920.017"
"          225.150  0.1672  998.211"
"          Peak outflow            0.039      c.m/sec"
"          Maximum level          224.939  metre"
"          Maximum storage        658.328  c.m"
"          Centroidal lag         13.073  hours"
"          0.535      0.535      0.039      0.000 c.m/sec"

```

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        \\os-2012r2\Users_Private\ewebb\Documents\
"                                          MIDUSS\221210\July 2022"
"          Output filename:                   Ex. and Post 100 yr.out"
"          Licensee name:                     gmbp"
"          Company                            "
"          Date & Time last used:            7/18/2022 at 8:26:32 AM"
" 31          TIME PARAMETERS"
"          5.000 Time Step"
"          210.000 Max. Storm length"
"          360.000 Max. Hydrograph"
" 32          STORM Chicago storm"
"          1 Chicago storm"
"          2171.754 Coefficient A"
"          8.303 Constant B"
"          0.867 Exponent C"
"          0.375 Fraction R"
"          210.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                   202.862 mm/hr"
"          Total depth                         71.271 mm"
"          6 100hyd Hydrograph extension used in this file"
" 33          CATCHMENT 10"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          10 Entire Site - Existing Conditions"
"          0.000 % Impervious"
"          2.010 Total Area"
"          200.000 Flow length"
"          2.900 Overland Slope"
"          2.010 Pervious Area"
"          200.000 Pervious length"
"          2.900 Pervious slope"
"          0.000 Impervious Area"
"          200.000 Impervious length"
"          2.900 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          71.000 Pervious SCS Curve No."
"          0.313 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          10.375 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"

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"		0.091	0.000	0.000	0.000	c.m/sec"
"	Catchment 10		Pervious	Impervious	Total Area	"
"	Surface Area	2.010		0.000	2.010	hectare"
"	Time of concentration	42.231		4.735	42.230	minutes"
"	Time to Centroid	159.260		101.449	159.260	minutes"
"	Rainfall depth	71.271		71.271	71.271	mm"
"	Rainfall volume	1432.55		0.00	1432.55	c.m"
"	Rainfall losses	48.990		5.636	48.990	mm"
"	Runoff depth	22.281		65.635	22.281	mm"
"	Runoff volume	447.85		0.00	447.85	c.m"
"	Runoff coefficient	0.313		0.000	0.313	"
"	Maximum flow	0.091		0.000	0.091	c.m/sec"
" 40	HYDROGRAPH Start - New Tributary"					
"	2	Start - New Tributary"				
"		0.091	0.000	0.000	0.000"	
" 33	CATCHMENT 100"					
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	100	Entire Site - Post-Dev."				
"	75.000	% Impervious"				
"	2.010	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.503	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.507	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	71.000	Pervious SCS Curve No."				
"	0.315	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	10.375	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.907	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.676	0.000	0.000	0.000	c.m/sec"
"	Catchment 100		Pervious	Impervious	Total Area	"
"	Surface Area	0.503		1.507	2.010	hectare"
"	Time of concentration	11.859		1.330	2.422	minutes"
"	Time to Centroid	122.661		96.344	99.075	minutes"
"	Rainfall depth	71.271		71.271	71.271	mm"
"	Rainfall volume	358.14		1074.41	1432.55	c.m"
"	Rainfall losses	48.815		6.631	17.177	mm"
"	Runoff depth	22.456		64.641	54.094	mm"
"	Runoff volume	112.84		974.46	1087.30	c.m"

"	Runoff coefficient	0.315	0.907	0.759	"
"	Maximum flow	0.049	0.670	0.676	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.676 0.676 0.000 0.000"				
" 54	POND DESIGN"				
"	0.676 Current peak flow	c.m/sec"			
"	0.356 Target outflow	c.m/sec"			
"	1087.3 Hydrograph volume	c.m"			
"	26. Number of stages"				
"	223.900 Minimum water level	metre"			
"	225.150 Maximum water level	metre"			
"	223.900 Starting water level	metre"			
"	0 Keep Design Data: 1 = True; 0 = False"				
"	Level Discharge	Volume"			
"	223.900 0.000	0.000"			
"	223.950 1.01E-05	1.01E-05"			
"	224.000 2.01E-05	2.965"			
"	224.050 3.01E-05	6.662"			
"	224.100 4.01E-05	10.726"			
"	224.150 0.00050	15.155"			
"	224.200 0.00060	19.950"			
"	224.250 0.00120	25.112"			
"	224.300 0.00150	30.639"			
"	224.350 0.00220	36.533"			
"	224.400 0.00260	42.792"			
"	224.450 0.00340	49.418"			
"	224.500 0.00390	56.410"			
"	224.550 0.00430	90.135"			
"	224.600 0.00470	158.292"			
"	224.650 0.00510	227.871"			
"	224.700 0.00540	298.882"			
"	224.750 0.00570	371.339"			
"	224.800 0.00600	445.252"			
"	224.850 0.00620	520.635"			
"	224.900 0.03810	597.500"			
"	224.950 0.03910	675.859"			
"	225.000 0.04020	755.723"			
"	225.050 0.04120	837.105"			
"	225.100 0.08320	920.017"			
"	225.150 0.1672	998.211"			
"	Peak outflow	0.041	c.m/sec"		
"	Maximum level	225.045	metre"		
"	Maximum storage	828.663	c.m"		
"	Centroidal lag	11.687	hours"		
"	0.676 0.676 0.041 0.000	c.m/sec"			